

Load on  $l_3$ . (By symmetry.)

$$M_2 = + \frac{l}{56} \{ 4 K_2 - K_1 \} \quad M_3 = \frac{-l}{14} \{ 4 K_2 - K_1 \}$$

$$M_4 = \frac{-l}{14} \left\{ \frac{15}{4} K_1 - K_2 \right\}$$

Load on  $l_4$ .

$$M_2 = - \frac{1}{56} l K_2. \quad M_3 = + \frac{1}{14} l K_2. \quad M_4 = \frac{-15}{56} l K_2.$$

INFLUENCE LINE OF  $M_2$ .

$$\text{Load on } l_1. \quad M_2 = \frac{-15}{56} l K_1. \quad \int_{k=0}^{k=1} M_2 = \frac{-15}{224} Wl.$$

$$\text{Load on } l_2. \quad M_2 = \frac{-l}{14} \left\{ \frac{15}{4} K_2 - K_1 \right\} \quad \int_{k=0}^{k=1} M_2 = \frac{-11}{224} Wl.$$

$$\text{Load on } l_3. \quad M_2 = \frac{l}{56} \{ 4 K_2 - K_1 \} \quad \int_{k=0}^{k=1} M_2 = \frac{3}{224} Wl.$$

$$\text{Load on } l_4. \quad M_2 = \frac{-1}{56} l K_2. \quad \int_{k=0}^{k=1} M_2 = \frac{-1}{224} Wl.$$

INFLUENCE LINE OF  $M_3$ .

$$\text{Load on } l_1. \quad M_3 = + \frac{1}{14} l K_1. \quad \int_{k=0}^{k=1} M_3 = \frac{1}{56} Wl.$$

$$\text{Load on } l_2. \quad M_3 = \frac{-l}{14} \{ 4 K_1 - K_2 \} \quad \int_{k=0}^{k=1} M_3 = \frac{-3}{56} Wl.$$

$$\text{Load on } l_3. \quad M_3 = \frac{-l}{14} \{ 4 K_2 - K_1 \} \quad \int_{k=0}^{k=1} M_3 = \frac{-3}{56} Wl.$$

$$\text{Load on } l_4. \quad M_3 = + \frac{1}{14} l K_2. \quad \int_{k=0}^{k=1} M_3 = + \frac{1}{56} Wl.$$

The curves of  $M_2$  and  $M_3$  are shown in Figs. 9a and 9b.

Table 9a.—SEE FIGURES 9a AND 9b.

<i>k</i>	$\frac{M_2}{l}$				$\frac{M_3}{l}$			
	LOAD ON				LOAD ON			
	<i>l</i> <sub>1</sub>	<i>l</i> <sub>2</sub>	<i>l</i> <sub>3</sub>	<i>l</i> <sub>4</sub>	<i>l</i> <sub>1</sub>	<i>l</i> <sub>2</sub>	<i>l</i> <sub>3</sub>	<i>l</i> <sub>4</sub>
	—	—	+	—	+	—	—	+
·05	·0134	·0212	·0057	·0017	·0036	·0076	·0229	·0066
·10	·0265	·0387	·0106	·0031	·0071	·0161	·0418	·0122
·15	·0393	·0527	·0142	·0042	·0105	·0250	·0569	·0169
·20	·0514	·0634	·0171	·0051	·0137	·0343	·0686	·0206
·25	·0628	·0711	·0192	·0059	·0167	·0435	·0770	·0234
·30	·0731	·0761	·0206	·0064	·0195	·0525	·0825	·0255
·35	·0823	·0786	·0213	·0067	·0219	·0608	·0852	·0268
·40	·0900	·0789	·0214	·0069	·0240	·0686	·0857	·0274
·45	·0961	·0771	·0210	·0068	·0256	·0751	·0840	·0274
·50	·1004	·0737	·0201	·0067	·0268	·0804	·0804	·0268
·55	·1027	·0687	·0188	·0064	·0274	·0840	·0751	·0256
·60	·1029	·0626	·0171	·0060	·0274	·0857	·0686	·0240
·65	·1006	·0554	·0152	·0055	·0268	·0852	·0608	·0219
·70	·0956	·0476	·0131	·0049	·0255	·0825	·0525	·0195
·75	·0879	·0394	·0109	·0042	·0234	·0770	·0435	·0167
·80	·0771	·0309	·0086	·0034	·0206	·0686	·0343	·0137
·85	·0632	·0224	·0063	·0026	·0169	·0569	·0250	·0105
·90	·0458	·0143	·0040	·0018	·0122	·0418	·0161	·0071
·95	·0248	·0068	·0020	·0009	·0066	·0229	·0076	·0036
1·00	·0000	·0000	·0000	·0000	·0000	·0000	·0000	·0000

REACTION  $R_1$ .

$$\text{Load on } l_1 \quad R_1 = \frac{M_2}{l} + (1 - k) = (1 - k) - \frac{15}{56} K_1.$$

$$\begin{aligned} k &= 1 \\ \int R_1 &= + \frac{97}{224} W. \\ k &= 0 \end{aligned}$$

$$\text{Load on } l_2 \quad R_1 = \frac{M_2}{l} = - \frac{1}{14} \left\{ \frac{15}{4} K_2 - K_1 \right\}$$

$$\begin{aligned} k &= 1 \\ \int R_1 &= - \frac{11}{224} W. \\ k &= 0 \end{aligned}$$

$$\text{Load on } l_3 \quad R_1 = \frac{M_2}{l} = + \frac{1}{56} \left\{ 4K_2 - K_1 \right\}$$

$$\begin{aligned} k &= 1 \\ \int R_1 &= \frac{3}{224} W_1. \\ k &= 0 \end{aligned}$$

$$\text{Load on } l_4. \quad R_1 = \frac{M_2}{l} = -\frac{1}{56} K_2.$$

$$\int_{k=0}^{k=1} R_1 = -\frac{1}{224} W.$$

For Influence Line curve, see Fig. 9c.

#### REACTION $R_2$ .

$$\text{Load on } l_1 \quad R_2 = (2 - k) - 2R_1 + \frac{M_3}{l} = k + \frac{17}{28} K_1.$$

$$\int_{k=0}^{k=1} R_2 = \frac{73}{112} W.$$

$$\text{Load on } l_2 \quad R_2 = (1 - k) - 2R_1 + \frac{M_3}{l} = (1 - k) + \frac{1}{28} \{17K_2 - 12K_1\}$$

$$\int_{k=0}^{k=1} R_2 = \frac{61}{112} W.$$

$$\text{Load on } l_3 \quad R_2 = -2R_1 + \frac{M_3}{l} = -\frac{3}{28} \{4K_2 - K_1\}$$

$$\int_{k=0}^{k=1} R_2 = \frac{-9}{112} W.$$

$$\text{Load on } l_4 \quad R_2 = -2R_1 + \frac{M_3}{l} = \frac{+3}{28} K_2.$$

$$\int_{k=0}^{k=1} R_2 = \frac{+3}{112} W.$$

The Influence Line curve is shewn in Fig. 9d.

#### REACTION $R_3$ .

$$\text{Load on } l_1 \quad R_3 = (3 - k) - 3R_1 - 2R_2 + \frac{M_4}{l} = \frac{-3}{7} K_1.$$

$$\int_{k=0}^{k=1} R_3 = \frac{-3}{28} W.$$

$$\text{Load on } l_2 \quad R_3 = (2-k) - 3R_1 - 2R_2 + \frac{M_4}{l} = k - \frac{3}{7}K_2 + \frac{5}{7}K_1.$$

$$\begin{aligned} k &= 1 \\ \int R_3 &= \frac{+4}{7} W. \\ k &= 0 \end{aligned}$$

$$\text{Load on } l_3 \quad R_3 = (1-k) - 3R_1 - 2R_2 + \frac{M_4}{l} = (1-k) + \frac{5}{7}K_2 - \frac{3}{7}K_1.$$

$$\begin{aligned} k &= 1 \\ \int R_3 &= \frac{+4}{7} W. \\ k &= 0 \end{aligned}$$

$$\text{Load on } l_4 \quad R_3 = -3R_1 - 2R_2 + \frac{M_4}{l} = -\frac{3}{7}K_2.$$

$$\begin{aligned} k &= 1 \\ \int R_3 &= -\frac{3}{28} W. \\ k &= 0 \end{aligned}$$

The last two could be deduced by symmetry.

The Influence Line curve is shewn in Fig. 9e.

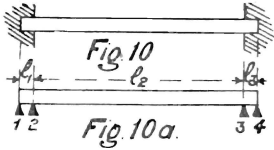
There is no need to work out  $R_4$  and  $R_5$  on account of symmetry.

Table 9c.—SEE FIGURES 9c, 9d AND 9e.

Reactions at 1, 2 and 3.

k	R <sub>1</sub>				R <sub>2</sub>				R <sub>3</sub>			
	LOAD ON				LOAD ON				LOAD ON			
	l <sub>1</sub>	l <sub>2</sub>	l <sub>3</sub>	l <sub>4</sub>	l <sub>1</sub>	l <sub>2</sub>	l <sub>3</sub>	l <sub>4</sub>	l <sub>1</sub>	l <sub>2</sub>	l <sub>3</sub>	l <sub>4</sub>
	+	-	+	-	+	+	-	+	-	+	+	-
·05	·9366	·0212	·0057	·0017	·0803	·9849	·0344	·0099	·0214	·0459	·9948	·0397
·10	·8735	·0387	·0106	·0031	·1601	·9614	·0627	·0183	·0424	·0974	·9797	·0733
·15	·8107	·0527	·0142	·0042	·2390	·9304	·0853	·0253	·0628	·1536	·9557	·1011
·20	·7486	·0634	·0171	·0051	·3166	·8926	·1028	·0309	·0823	·2137	·9234	·1234
·25	·6872	·0711	·0192	·0059	·3923	·8487	·1154	·0352	·1005	·2768	·8839	·1406
·30	·6269	·0761	·0206	·0064	·4658	·7997	·1238	·0382	·1170	·3420	·8380	·1530
·35	·5677	·0786	·0213	·0067	·5365	·7463	·1278	·0402	·1316	·4085	·7865	·1609
·40	·5100	·0789	·0214	·0069	·6040	·6891	·1286	·0411	·1440	·4754	·7303	·1646
·45	·4539	·0771	·0210	·0068	·6679	·6291	·1260	·0411	·1538	·5420	·6702	·1644
·50	·3996	·0737	·0201	·0067	·7277	·5670	·1206	·0402	·1607	·6071	·6071	·1607
·55	·3473	·0687	·0188	·0064	·7829	·5035	·1127	·0385	·1644	·6702	·5420	·1538
·60	·2971	·0626	·0171	·0060	·8331	·4394	·1029	·0360	·1646	·7303	·4754	·1440
·65	·2494	·0554	·0152	·0055	·8779	·3756	·0912	·0329	·1609	·7865	·4085	·1316
·70	·2044	·0476	·0131	·0049	·9167	·3128	·0788	·0292	·1530	·8380	·3420	·1170
·75	·1621	·0394	·0109	·0042	·9492	·2517	·0652	·0251	·1406	·8839	·2768	·1005
·80	·1229	·0309	·0086	·0034	·9749	·1932	·0514	·0206	·1234	·9234	·2137	·0823
·85	·0868	·0224	·0063	·0026	·9932	·1379	·0376	·0157	·1011	·9557	·1536	·0628
·90	·0542	·0143	·0040	·0018	1·0038	·0868	·0241	·0106	·0733	·9797	·0974	·0424
·95	·0252	·0068	·0020	·0009	1·0062	·0406	·0115	·0053	·0397	·9948	·0459	·0214
1·00	·0000	·0000	·0000	·0000	1·0000	·0000	·0000	·0000	·0000	1·0000	·0000	·0000

**Case 10.—BEAM OF ONE SPAN FIXED AT ENDS.** (FIG. 10.)



Bending Moments.

Referring to Fig. 10a.

Load on  $l_2$ .

$$M_1 l_1 + 2M_2 (l_1 + l_2) + M_3 l_2 = -l_2^2 K_2$$

$$M_2 l_2 + 2M_3 (l_2 + l_3) + M_4 l_3 = -l_2^2 K_1$$

Here  $l_1 = l_3 = 0$  and  $M_1 = M_2$  also  $M_3 = M_4$

$$\therefore 2M_2 + M_3 = -l_2^2 K_2$$

$$M_2 + 2M_3 = -l_2^2 K_1$$

$$\therefore M_1 = M_2 = \frac{1}{3} l (K_1 - 2K_2), \quad M_3 = M_4 = \frac{1}{3} l (K_2 - 2K_1).$$

$$k = 1$$

$$\int M_2 = -\frac{1}{12} Wl.$$

$$k = 0$$

Curve is drawn in Fig. 10b.

**Table 10b.—Bending Moments at 2.**  $\frac{M_2}{l}$

$k$	0	1	2	3	4	5	6	7	8	9
	—	—	—	—	—	—	—	—	—	—
0	·0000	·0098	·0192	·0282	·0369	·0451	·0530	·0605	·0577	·0745
1	·0810	·0871	·0929	·0984	·1035	·1084	·1129	·1171	·1210	·1247
2	·1280	·1311	·1338	·1364	·1386	·1406	·1424	·1439	·1452	·1462
3	·1470	·1476	·1480	·1481	·1481	·1479	·1475	·1469	·1461	·1451
4	·1440	·1427	·1413	·1397	·1380	·1361	·1341	·1320	·1298	·1275
5	·1250	·1225	·1198	·1171	·1143	·1114	·1084	·1054	·1023	·0992
6	·0960	·0928	·0895	·0862	·0830	·0796	·0763	·0730	·0696	·0663
7	·0630	·0597	·0564	·0532	·0500	·0469	·0438	·0407	·0378	·0348
8	·0320	·0292	·0266	·0240	·0215	·0191	·0169	·0147	·0127	·0108
9	·0090	·0074	·0059	·0046	·0034	·0024	·0015	·0009	·0004	·0001

Bending Moment at Centre.  $M_c$ .

$$\text{We have } R_2 \times l_2 + M_2 - (1 - k) l_2 = M_3$$

$$\therefore R_2 = (1 - k) + \left\{ M_3 - M_2 \right\} \div l$$

$$= (1 - k) + (K_2 - K_1)$$

$$\text{and } R_3 = k + (K_1 - K_2)$$

INFLUENCE LINE  $M_c$ .

$$M_c = R_2 \times \frac{l}{2} - \left( \frac{l}{2} - k \right) + M_2$$

$$\text{or } = R_3 \times \frac{l}{2} + M_3 \text{ from } k = 0 \text{ to } k = \frac{1}{2}$$

$$\text{and } = R_2 \times \frac{l}{2} + M_2 \text{ from } k = \frac{1}{2} \text{ to } k = 1$$

Left half  $k = 0$  to  $k = \frac{1}{2}$

$$\begin{aligned} M_c &= \left\{ k + (K_1 - K_2) \right\} \frac{l}{2} + \frac{1}{3} \left\{ K_2 - 2K_1 \right\} l \\ &= \left\{ \frac{k}{2} - \frac{1}{6} (K_1 + K_2) \right\} l \end{aligned}$$

Right half  $k = \frac{1}{2}$  to  $k = 1$

$$\begin{aligned} M_c &= \left\{ \frac{1-k}{2} - \frac{1}{6} (K_1 + K_2) \right\} l \\ &\quad k = 1 \quad k = \frac{1}{2} \\ &\bullet \int_{k=0}^1 M_c = 2 \int_{k=0}^{\frac{1}{2}} \frac{k}{2} - \frac{1}{6} \int \left\{ K_1 + K_2 \right\} \\ &= \left\{ \frac{1}{8} - \frac{1}{12} \right\} Wl = \frac{Wl}{24} \end{aligned}$$

The curve is shewn in Fig. 10c.

For uniform loading throughout and for beam loaded in the centre, the upward B. Mt. (not the ultimate B. Mt.) is the same as for a beam with free ends, but for any partial loading the upward and also the ultimate B. Mt. are different, and the shears also are different. These points will be referred to again later.

**Table 10c.—Bending Moment at Centre.**

$k$	0	1	2	3	4	5	6	7	8	9
	+	+	+	+	+	+	+	+	+	+
0	·0000	·0000	·0002	·0004	·0008	·0012	·0018	·0024	·0032	·0040
1	·0050	·0060	·0072	·0084	·0098	·0112	·0128	·0144	·0162	·0180
2	·0200	·0220	·0242	·0264	·0288	·0312	·0338	·0364	·0392	·0420
3	·0450	·0480	·0512	·0544	·0578	·0612	·0648	·0684	·0722	·0760
4	·0800	·0840	·0882	·0924	·0968	·1012	·1058	·1104	·1152	·1200
5	·1250	·1200	·1152	·1104	·1058	·1012	·0968	·0924	·0882	·0840
6	·0800	·0760	·0722	·0684	·0648	·0612	·0578	·0544	·0512	·0480
7	·0450	·0420	·0392	·0364	·0338	·0312	·0288	·0264	·0242	·0220
8	·0200	·0180	·0162	·0144	·0128	·0112	·0098	·0084	·0072	·0060
9	·0050	·0040	·0032	·0024	·0018	·0012	·0008	·0004	·0002	·0000

INFLUENCE LINE OF  $R_2$ .

$$R_2 = (1 - k) + (K_2 - K_1) \int_{k=0}^k R_2 = \frac{1}{2}.$$

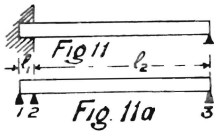
Curve is shewn in Fig. 10d.

Table 10d.—Reaction at End.

$k$	0	1	2	3	4	5	6	7	8	9
0	1·000	·9997	·9988	·9973	·9954	·9927	·9896	·9859	·9818	·9771
1	·9720	·9663	·9602	·9537	·9466	·9393	·9314	·9231	·9144	·9055
2	·8960	·8863	·8760	·8657	·8548	·8437	·8324	·8207	·8088	·7965
3	·7840	·7713	·7584	·7451	·7318	·7183	·7044	·6907	·6766	·6623
4	·6480	·6335	·6190	·6043	·5896	·5747	·5598	·5449	·5300	·5149
5	·5000	·4851	·4700	·4551	·4402	·4253	·4104	·3957	·3810	·3665
6	·3520	·3377	·3134	·3093	2956	·2817	·2682	·2549	·2416	·2287
7	·2160	·2035	·1912	·1793	·1676	·1563	·1452	·1343	·1240	·1137
8	·1040	·0945	·0856	·0769	·0686	·0607	·0534	·0463	·0398	·0337
9	·0280	·0229	·0182	·0141	·0104	·0073	·0046	·0027	·0012	·0003

## Case 11.—BEAM FIXED AT ONE END AND FREE AT OTHER.

(FIG. 11.)



BENDING MOMENTS at end.

Referring to Fig. 11a.  $M_3 = M_4 = 0$ .

$$M_1 l_1 + 2M_2 (l_1 + l_2) + M_3 l_2 = -\frac{1}{2} W l^2 K_2.$$

Here  $M_3 = M_4 = 0$ , also  $M_1 = M_2$  and  $l_1 = l_3 = 0$ 

$$\therefore 2M_2 l_2 = -\frac{1}{2} W l^2 K_2$$

$$M_2 = -\frac{1}{4} W l^2 K_2 \quad \int M_2 = -\frac{Wl^3}{8} K_2$$

Curve is shewn in Fig. 11b.

REACTIONS.

$$R_2 = (1 - k) - \frac{M_2}{l} \text{ since } M_3 = 0$$

$$= (1 - k) + \frac{1}{4} W l K_2 \quad \int R_2 = \frac{5}{8} W l$$

$$R_3 = k + \frac{M_2}{l} = k - \frac{1}{4} W l K_2$$

$$\int R_3 = \frac{3}{8} W l$$

Curves are shewn in Figs. 11c, 11d.

BENDING MOMENT at centre.

$$\begin{aligned}
 M_c &= R_3 \times \frac{l}{2} \text{ from } k = 0 \text{ to } k = \frac{1}{2}. \\
 \text{and} &= R_2 \times \frac{l}{2} + M_2 \text{ from } k = \frac{1}{2} \text{ to } k = 1. \\
 \therefore M_c &= \left\{ k - \frac{1}{2} K_2 \right\} \frac{l}{2} \text{ from } k = 0 \text{ to } k = \frac{1}{2} \\
 \text{and} &= \left\{ (1 - k) - \frac{1}{2} K_2 \right\} \frac{l}{2} \text{ from } k = \frac{1}{2} \text{ to } k = 1. \\
 \int_{k=0}^{k=1} M_c &= \frac{1}{16} Wl.
 \end{aligned}$$

Curve is shewn in Fig. 11e.

B. Mts. and Shears at any point can be determined from the Reactions and B. Mts. at end in a similar manner to that by which  $M_c$  was deduced.

Table 11.—Bending Moments and Reactions.

Figs. 11b to 11e.

$k$	$\frac{M_2}{l}$	$\frac{M_c}{l}$	$R_2$	$R_3$	$k$	$\frac{M_2}{l}$	$\frac{M_c}{l}$	$R_2$	$R_3$
	—	+	+	+		—	+	+	+
·05	·0463	·0019	·9963	·0037	·55	·1795	·1291	·6295	·3705
·10	·0855	·0073	·9855	·0145	·60	·1680	·1040	·5680	·4320
·15	·1180	·0160	·9680	·0320	·65	·1536	·0812	·5036	·4964
·20	·1440	·0280	·9440	·0560	·70	·1365	·0608	·4365	·5635
·25	·1641	·0430	·9141	·0859	·75	·1172	·0430	·3672	·6328
·30	·1785	·0608	·8785	·1215	·80	·0960	·0280	·2960	·7040
·35	·1877	·0812	·8377	·1623	·85	·0733	·0160	·2233	·7767
·40	·1920	·1040	·7920	·2080	·90	·0495	·0073	·1495	·8505
·45	·1918	·1291	·7418	·2582	·95	·0250	·0019	·0750	·9250
·50	·1875	·1563	·6875	·3125	1·00	·0000	·0000	·0000	1·0000

From the above it will be noticed that for a central load the B. Mt. at centre is reduced in ratio  $\frac{1}{2} \frac{5}{50}$  by fixing, *i.e.*, B. Mt. is about ·624 what it would be if the end were not fixed.

For a uniformly distributed load throughout, the B. Mt. at centre is reduced by  $\frac{1}{2}$ . For partial loading the reduction of B. Mt. is readily deduced from the Reactions and B. Mts. at end.

NOTE.—It may be remarked here that in all cases of summation for partial loading, it follows that since  $k$  is merely a multiplying factor of  $l$ , the area of the curve must be found between the limits specified and multiplied by  $wl^2$  where  $w$  is the load per unit length (at first



sight it might appear as if the area ought to be multiplied by the load on the portion between the limits). Thus if  $W_a$  is the load on the portion between  $k = k_1$  and  $k = k_2$ .

$$\text{B. Mt.} = \frac{W_a}{k_2 - k_1} l \int_{k_1}^{k_2} F(k) dk.$$

$$\text{Similarly Reactions} = \frac{W_a}{k_2 - k_1} \int_{k_1}^{k_2} F_1(k) dk.$$

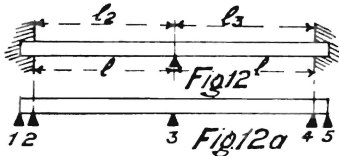
This follows thus  $\int M = \int F(k) \times l \times dW$ .

$$\text{and } dW = \frac{W_a d(kl)}{k_2 l - k_1 l} = \frac{W_a}{k_2 - k_1} dk.$$

$$\therefore \int M = \int \frac{W_a}{k_2 - k_1} F(k) dk = \frac{W_a}{k_2 - k_1} \int_{k_1}^{k_2} F(k) dk.$$

### Case 12.—BEAM OF TWO EQUAL SPANS FIXED AT BOTH ENDS.

(FIG. 12.)



BENDING MOMENT at supports.

Load on  $l_2$ .

$$2M_2 + M_3 = -K_2 l$$

$$M_2 + 4M_3 + M_4 = -K_1 l$$

$$M_3 + 2M_4 = 0.$$

$$\therefore M_2 = -\frac{1}{12} (7K_2 - 2K_1) l \quad \int_{k=0}^{k=1} M_2 = -\frac{5}{48} Wl$$

$$M_3 = -\frac{1}{6} (2K_1 - K_2) l \quad \int_{k=0}^{k=1} M_3 = -\frac{1}{24} Wl$$

$$M_4 = \frac{1}{12} (2K_1 - K_2) l \quad \int_{k=0}^{k=1} M_4 = \frac{1}{48} Wl$$

Load on  $l_3$  (by symmetry).

$$\begin{aligned}
 M_2 &= \frac{1}{12} (2K_2 - K_1)l & \int_{k=0}^{k=1} M_2 &= \frac{1}{48} Wl \\
 M_3 &= -\frac{1}{6} (2K_2 - K_1)l & \int_{k=0}^{k=1} M_3 &= -\frac{1}{24} Wl \\
 M_4 &= \frac{1}{12} (2K_2 - 7K_1)l & \int_{k=0}^{k=1} M_4 &= -\frac{5}{48} Wl.
 \end{aligned}$$

The curves for  $M_2$  and  $M_3$  are shewn in Figs. 12*b*, 12*c*.

REACTIONS.

Load on  $l_2$ .

$$\begin{aligned}
 R_2 &= \frac{M_3 - M_2}{l} + (1 - k) = (1 - k) - \frac{1}{4} \{ 2K_1 - 3K_2 \} \\
 R_4 &= -\frac{M_3 - M_4}{l} = -\frac{1}{4} \{ 2K_1 - K_2 \} & \int_{k=0}^{k=1} R_4 &= -\frac{1}{16} W \\
 R_3 &= \frac{M_2 - M_4}{l} - 2R_4 + k = k + \{ K_1 - K_2 \} & \int_{k=0}^{k=1} R_3 &= \frac{1}{2} W
 \end{aligned}$$

Load on  $l_3$  (by symmetry).

$$\begin{aligned}
 R_4 &= k - \frac{1}{4} \{ 2K_2 - 3K_1 \} & \int_{k=0}^{k=1} R_4 &= \frac{9}{10} W \\
 R_2 &= -\frac{1}{4} \{ 2K_2 - K_1 \} & \int_{k=0}^{k=1} R_2 &= -\frac{1}{16} W \\
 R_3 &= (1 - k) + \{ K_2 - K_1 \} & \int_{k=0}^{k=1} R_3 &= \frac{1}{2} W.
 \end{aligned}$$

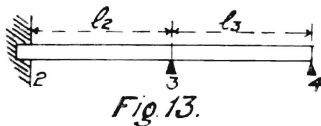
The curves for  $R_2$  and  $R_3$  are shewn Figs. 12*d*, 12*e*.

**Table 12.—Bending Moments and Reactions. (Figs. 12—12c).**

k	M <sub>2</sub>		M <sub>3</sub>		R <sub>2</sub>		R <sub>3</sub>	
	l		l					
	LOAD ON l <sub>2</sub>	LOAD ON l <sub>3</sub>	LOAD ON l <sub>2</sub>	LOAD ON l <sub>3</sub>	LOAD ON l <sub>2</sub>	LOAD ON l <sub>3</sub>	LOAD ON l <sub>2</sub>	LOAD ON l <sub>3</sub>
.05	-.0457	+.0113	-.0012	-.0226	+.9945	-.0338	+.0073	+.9927
.10	.0863	.0208	.0045	.0415	.9788	.0608	.0280	.9720
.15	.1132	.0271	.0096	.0542	.9536	.0813	.0607	.9393
.20	.1360	.0320	.0160	.0640	.9200	.0960	.1040	.8960
.25	.1523	.0352	.0235	.0703	.8789	.1055	.1563	.8437
.30	.1628	.0368	.0315	.0735	.8313	.1103	.2160	.7840
.35	.1678	.0370	.0398	.0739	.7780	.1109	.2817	.7183
.40	.1680	.0360	.0480	.0720	.7200	.1080	.3520	.6480
.45	.1639	.0340	.0557	.0681	.6583	.1021	.4253	.5747
.50	.1563	.0313	.0625	.0625	.5938	.0938	.5000	.5000
.55	.1454	.0279	.0681	.0557	.5274	.0836	.5747	.4253
.60	.1320	.0240	.0720	.0480	.4600	.0720	.6480	.3520
.65	.1166	.0199	.0739	.0398	.3926	.0597	.7183	.2817
.70	.0998	.0158	.0735	.0315	.3263	.0473	.7840	.2160
.75	.0821	.0117	.0703	.0235	.2618	.0352	.8437	.1563
.80	.0640	.0080	.0640	.0160	.2000	.0240	.8960	.1040
.85	.0379	.0048	.0542	.0096	.1420	.0143	.9393	.0607
.90	.0293	.0023	.0415	.0045	.0887	.0068	.9720	.0280
.95	.0137	.0006	.0226	.0012	.0411	.0018	.9927	.0073
1.00	.0000	.0000	.0000	.0000	.0000	.0000	1.0000	.0000

**Case 13.—BEAM OF TWO EQUAL SPANS FIXED AT ONE END, FREE AT OTHER. (FIG. 13.)**

BENDING MOMENTS.



Load on l<sub>2</sub>.  
 $2M_2 + M_3 = -K_2 l$   
 $M_2 + 4M_3 = -K_1 l$   
 $\therefore M_2 = -\frac{1}{7} \{4K_2 - K_1\} l$   
 $M_3 = -\frac{1}{7} \{2K_1 - K_2\} l$

Load on l<sub>3</sub>.  
 $2M_2 + M_3 = 0$   
 $M_2 + 4M_3 = -K_2 l$   
 $k = 1$   
 $\therefore M_2 = \frac{1}{7} K_2 l \int_{k=0}^k M_2 = \frac{1}{28} Wl$   $M_3 = -\frac{3}{7} K_2 l \int_{k=0}^k M_3 = -\frac{1}{14} Wl$

The curves for these are shown Figs. 13a, 13b.

REACTIONS.

Load on l<sub>2</sub>.  
 $R_2 = \frac{M_3 - M_2}{l} + (1 - k) = (1 - k) + \frac{1}{7} \{5K_2 - 3K_1\} \int_{k=0}^k R_2 = \frac{4}{7} W$

$$R_4 = \frac{M_3}{l} = -\frac{1}{7} \{ 2K_1 - K_2 \} \quad \int_{k=0}^{k=1} R_4 = -\frac{1}{28} W$$

$$R_3 = k + \frac{M_2}{l} - 2R_4 = k + \frac{1}{7} \{ 5K_1 - 6K_2 \} \quad \int_{k=0}^{k=1} R_3 = \frac{13}{28} W$$

Load on  $l_3$ .

$$R_4 = \frac{M_3}{l} + k = k - \frac{3}{7} K_2. \quad \int_{k=0}^{k=1} R_4 = \frac{3}{7} W$$

$$R_3 = \frac{M_2}{l} - 2R_4 + (1+k) = (1-k) + \frac{5}{7} K_2. \quad \int_{k=0}^{k=1} R_3 = \frac{19}{28} W$$

$$R_2 = \frac{M_3 - M_2}{l} = -\frac{3}{7} K_2. \quad \int_{k=0}^{k=1} R_2 = -\frac{3}{28} W$$

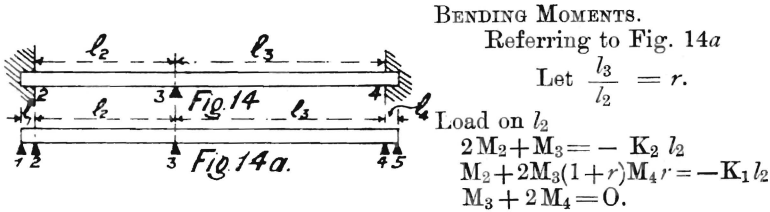
The curves for these are shewn Figs. 13c, 13d, 13e.

**Table 13.—Bending Moments and Reactions.** (Figs. 13—13e.)

k	$\frac{M_2}{l}$		$\frac{M_3}{l}$		R <sub>2</sub>		R <sub>3</sub>		R <sub>4</sub>	
	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$
·05	—	+	—	—	+	—	+	+	—	+
·10	·0836	·0244	·0039	·0489	·9797	·0733	·0241	1·0221	·0039	·0511
·15	·1139	·0337	·0082	·0674	·9557	·1011	·0525	1·0185	·0082	·0826
·20	·1371	·0411	·0137	·0823	·9234	·1234	·0903	1·0057	·0137	·1177
·25	·1540	·0469	·0201	·0937	·8839	·1406	·1362	·9844	·0201	·1563
·30	·1650	·0510	·0270	·1020	·8380	·1530	·1890	·9550	·0270	·1980
·35	·1706	·0536	·0341	·1073	·7865	·1609	·2476	·9181	·0341	·2427
·40	·1714	·0549	·0411	·1097	·7303	·1646	·3109	·8743	·0411	·2903
·45	·1679	·0548	·0477	·1096	·6702	·1644	·3776	·8240	·0477	·3404
·50	·1607	·0536	·0536	·1071	·6071	·1607	·4464	·7679	·0536	·3929
·55	·1503	·0513	·0583	·1025	·5420	·1538	·5164	·7064	·0583	·4475
·60	·1371	·0480	·0617	·0960	·4754	·1440	·5863	·6400	·0617	·5040
·65	·1219	·0439	·0634	·0877	·4085	·1316	·6549	·5694	·0634	·5623
·70	·1050	·0390	·0630	·0780	·3420	·1170	·7210	·4950	·0630	·6220
·75	·0871	·0335	·0603	·0670	·2768	·1005	·7834	·4174	·0603	·6830
·80	·0686	·0274	·0549	·0549	·2137	·0823	·8411	·3371	·0549	·7451
·85	·0501	·0209	·0465	·0419	·1536	·0628	·8928	·2547	·0465	·8081
·90	·0521	·0141	·0347	·0283	·0974	·0424	·9373	·1707	·0347	·8717
·95	·0153	·0071	·0193	·0143	·0460	·0214	·9734	·0856	·0193	·9357
1·00	·0000	·0000	·0000	·0000	·0000	·0000	1·0000	·0000	·0000	1·0000

**Case 14.—BEAM OF TWO UNEQUAL SPANS FIXED AT ENDS.**

(FIG. 14.)



BENDING MOMENTS.

Referring to Fig. 14a

$$\text{Let } \frac{l_3}{l_2} = r.$$

Load on  $l_2$

$$2M_2 + M_3 = -K_2 l_2$$

$$M_2 + 2M_3(1+r)M_4 r = -K_1 l_2$$

$$M_3 + 2M_4 = 0.$$

$$\therefore M_2 = \frac{1}{6(1+r)} \{ 2K_1 - (4 + 3r) K_2 \} l_2.$$

$$M_3 = \frac{-1}{3(1+r)} \{ 2K_1 - K_2 \} l_2.$$

$$M_4 = \frac{+1}{6(1+r)} \{ 2K_1 - K_2 \} l_2.$$

Load on  $l_3$ .

Put  $K_1$  for  $K_2$  and *vice versa*, also  $r = \frac{1}{r}$  and interchange  $M_4$  and  $M_2$  also  $l_2$  and  $l_3$ , then

$$M_2 = \frac{+r}{6(1+r)} \{ 2K_2 - K_1 \} l_3$$

$$M_3 = \frac{-r}{3(1+r)} \{ 2K_2 - K_1 \} l_3.$$

$$M_4 = \frac{1}{6(1+r)} \{ 2rK_2 - (3+4r)K_1 \} l_3.$$

The curves and tables are worked out for  $r = 1.5$ . See Figs. 14b, 14c, 14d.

REACTIONS.

Load on  $l_2$ .

$$R_2 = \frac{M_3 - M_2}{l_2} + (1-k) = (1-k) - \frac{1}{2(1+r)} \{ 2K_1 - (r+2) K_2 \}$$

$$R_4 = \frac{M_3 - M_4}{l_3} = -\frac{1}{2r(1+r)} \{ 2K_1 - K_2 \}$$

$$\begin{aligned} R_3 &= \frac{M_2 - M_4}{l_2} - R_4(1+r) + k \\ &= k + \frac{1}{2r} \{ 2K_1 - (1+r) K_2 \} \end{aligned}$$

Load on  $l_3$  (by symmetry.)

$$R_2 = \frac{-r^2}{2(1+r)} \{2K_2 - K_1\}$$

$$R_4 = k - \frac{r}{2(1+r)} \left\{ 2K_2 - \left( \frac{1}{r} + 2 \right) K_1 \right\}$$

$$= k - \frac{1}{2(1+r)} \left\{ 2rK_2 - (1+2r)K_1 \right\}$$

$$R_3 = 1 - k + \frac{r}{2} \left\{ 2K_2 - \frac{1+r}{r} K_1 \right\}$$

$$= (1 - k) + \frac{1}{2} \left\{ 2rK_2 - (1+r)K_1 \right\}$$

The curves for these are shewn in Figs. 14e-14g. The table is worked out for  $r = 1.5$ .

Table 14.—Bending Moments and Reactions for  $r = 1.5$ . FIGS. 14—14g.

$k$	$\frac{M_2}{l_2}$	$\frac{M_2}{l_3}$	$\frac{M_3}{l_2}$	$\frac{M_3}{l_3}$	$\frac{M_4}{l_2}$	$\frac{M_4}{l_3}$	$R_2$		$R_3$		$R_4$	
	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$	Load on $l_2$	Load on $l_3$
	-	+	-	-	+	-	+	-	+	+	-	+
·1	·0837	·0243	·0036	·0486	·0018	·0252	·9801	·1094	·0235	1·0328	·0036	·0766
·2	·1376	·0384	·0128	·0768	·0064	·0576	·9248	·1728	·0880	·9920	·0128	·1808
·3	·1659	·0441	·0252	·0882	·0126	·0924	·8407	·1985	·1845	·8943	·0252	·3042
·4	·1728	·0432	·0384	·0864	·0192	·1248	·7344	·1944	·3040	·7560	·0384	·4384
·5	·1625	·0375	·0500	·0750	·0250	·1500	·6125	·1688	·4375	·5938	·0500	·5750
·6	·1392	·0288	·0576	·0576	·0288	·1632	·4816	·1296	·5760	·4240	·0576	·7056
·7	·1071	·0189	·0588	·0378	·0294	·1596	·3483	·0851	·7105	·2633	·0588	·8218
·8	·0704	·0096	·0512	·0192	·0256	·1344	·2192	·0432	·8320	·1280	·0512	·9152
·9	·0333	·0027	·0324	·0054	·0162	·0828	·1009	·0122	·9315	·0347	·0324	·9774
1·0	·0000	·0000	·0000	·0000	·0000	·0000	·0000	·0000	1·0000	·0000	·0000	1·0000

Case 15.—BEAM OF TWO UNEQUAL SPANS, FIXED AT ONE END AND FREE AT OTHER. (FIG. 15.)

BENDING MOMENTS. Let  $\frac{l_3}{l_2} = r$

Load on  $l_2$ .

$$2M_2 + M_3 = -K_2 l_2.$$

$$M_2 + 2M_3(1+r) = -K_1 l_2.$$

$$M_2 = -\frac{1}{4r+3} \left\{ 2(1+r)K_2 - K_1 \right\} l_2.$$

$$M_3 = -\frac{1}{4r+3} \left\{ 2K_1 - K_2 \right\} l_2.$$

